

## **Dynamics of a continuous media**

- In dynamics we study the cause of the motion of a continuum.
- We introduce the laws of motion and conservation of Mass Momentum and Angular Momentum.
- These conservation principles lead to the equations of motion and the definition of the stress tensors.

From the book: Mechanics of Continuous Media: an Introduction

J Botsis and M Deville, PPUR 2018.

Solutions: <https://www.epflpress.org/produit/908/9782889152810/mechanics-of-continuous-media>

# Continuum mechanics review: Dynamics

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## CONSERVATION LAWS

We consider a quantity  $\varphi(\mathbf{x}, t)$  occupying a volume  $\omega(t)$  or  $\omega$  of a body in motion with velocity  $\mathbf{v}(\mathbf{x}, t)$ .

To establish the conservation of a given quantity in spatial description:

$$I(t) = \int_{\omega} \varphi(\mathbf{x}, t) dx_1 dx_2 dx_3$$

the variation of the time derivative of that quantity is investigated:

$$\frac{DI(t)}{Dt} = \frac{d}{dt} \int_{\omega} \varphi(\mathbf{x}, t) dx_1 dx_2 dx_3$$

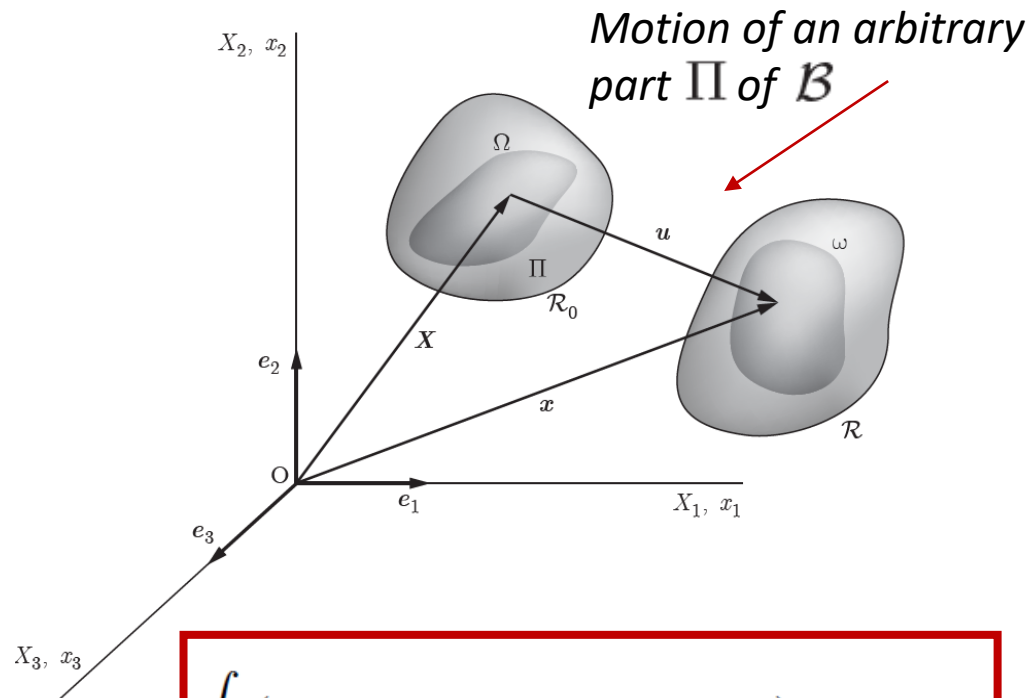
The analysis of this derivative results in the following integral known as **Raynolds Transport Theorem**:

$$\frac{DI(t)}{Dt} = \int_{\omega} \left( \frac{D\varphi(\mathbf{x}, t)}{Dt} + \varphi(\mathbf{x}, t) \nabla \cdot \mathbf{v}(\mathbf{x}, t) \right) dx_1 dx_2 dx_3$$

It plays an essential role (together with the divergence theorem) in the models of continuous media.

# Continuum mechanics review: Dynamics

## CONSERVATION OF MASS IN MATERIAL DESCRIPTION



$$\int_{\Omega} (J(\mathbf{X}, t) P(\mathbf{X}, t) - P_0(\mathbf{X})) dV = 0$$

or:  $J(\mathbf{X}, t) P(\mathbf{X}, t) = P_0(\mathbf{X})$

the medium is defined as **incompressible** when :  $J(\mathbf{X}, t) = 1$

initial mass  
initial mass density

$$m(\Omega) = \int_{\Omega} P_0(\mathbf{X}) dV = \int_{\Omega} P_0(\mathbf{X}) dX_1 dX_2 dX_3$$

current mass  
current mass density

$$m_t(\omega) = \int_{\omega} \rho(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t) dx_1 dx_2 dx_3 ,$$

**Principle of conservation of mass :**  $m_t(\omega) = m(\Omega)$

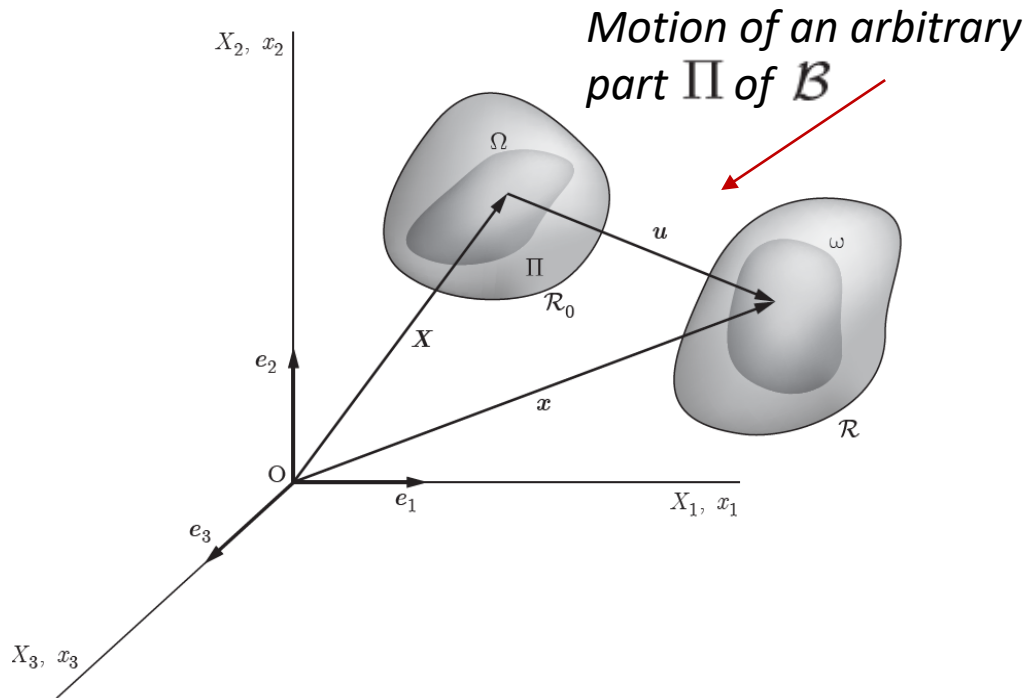
$$\int_{\omega} \rho(\mathbf{x}, t) dv = \int_{\Omega} P_0(\mathbf{X}) dV$$

We change variables on the LHS integral and considering:

$$P(\mathbf{X}, t) = \rho(\chi(\mathbf{X}, t), t)$$

# Continuum mechanics review: Dynamics

## CONSERVATION OF MASS IN SPATIAL DESCRIPTION



The material form of the conservation is used in solid mechanics while the spatial description in fluid mechanics.

The time derivative of:  $\int_{\omega} \rho(\mathbf{x}, t) dv = \int_{\Omega} P_0(\mathbf{X}) dV$

and use of the Reynolds theorem results in :

$$\frac{d}{dt} \int_{\omega} \rho(\mathbf{x}, t) dv = 0$$

or 
$$\int_{\omega} \left( \frac{D\rho(\mathbf{x}, t)}{Dt} + \rho(\mathbf{x}, t) \operatorname{div} \mathbf{v}(\mathbf{x}, t) \right) dv = 0$$

defined as the global form of the mass conservation in spatial description.

In local form the **principle of conservation of mass** is :

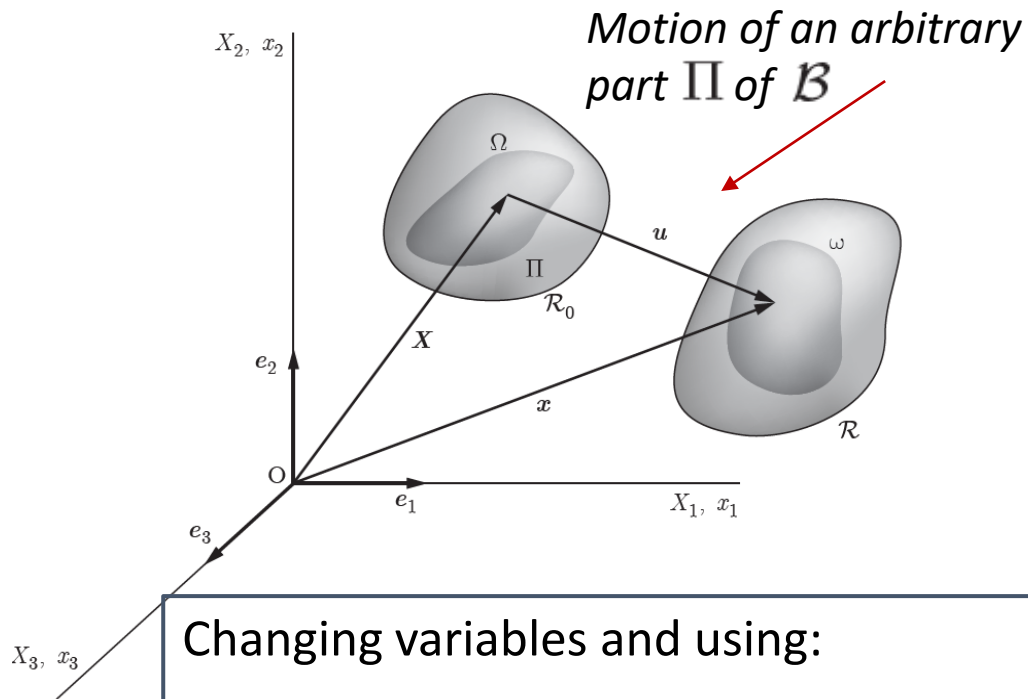
$$\frac{D\rho(\mathbf{x}, t)}{Dt} + \rho(\mathbf{x}, t) \operatorname{div} \mathbf{v}(\mathbf{x}, t) = 0$$

For an **incompressible material** :  $\operatorname{div} \mathbf{v} = \frac{\partial v_i}{\partial x_i} = 0$

and  $D\rho(\mathbf{x}, t)/Dt = 0$

# Continuum mechanics review: Dynamics

## BODY FORCES



Changing variables and using:

$$P(\mathbf{X}, t) = \rho(\chi(\mathbf{X}, t), t)$$

We obtain for the **volume force density**:

$$\mathbf{B}(\mathbf{X}, t) = \mathbf{b}(\chi(\mathbf{X}, t), t)$$

The time volume force acting on  $\Pi$  at time  $t$ :

$$\mathbf{f}^b(\omega, t) = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{b}(\mathbf{x}, t) dv$$

Here  $\mathbf{b}(\mathbf{x}, t)$  is a vector function defined in  $\mathcal{R}$  and called **spatial volume force density**.

The material form of this force is:

$$\mathbf{F}^b(\Omega, t) = \int_{\Omega} P_0(\mathbf{X}) \mathbf{B}(\mathbf{X}, t) dV$$

In both definitions they express the same quantity thus, we have:

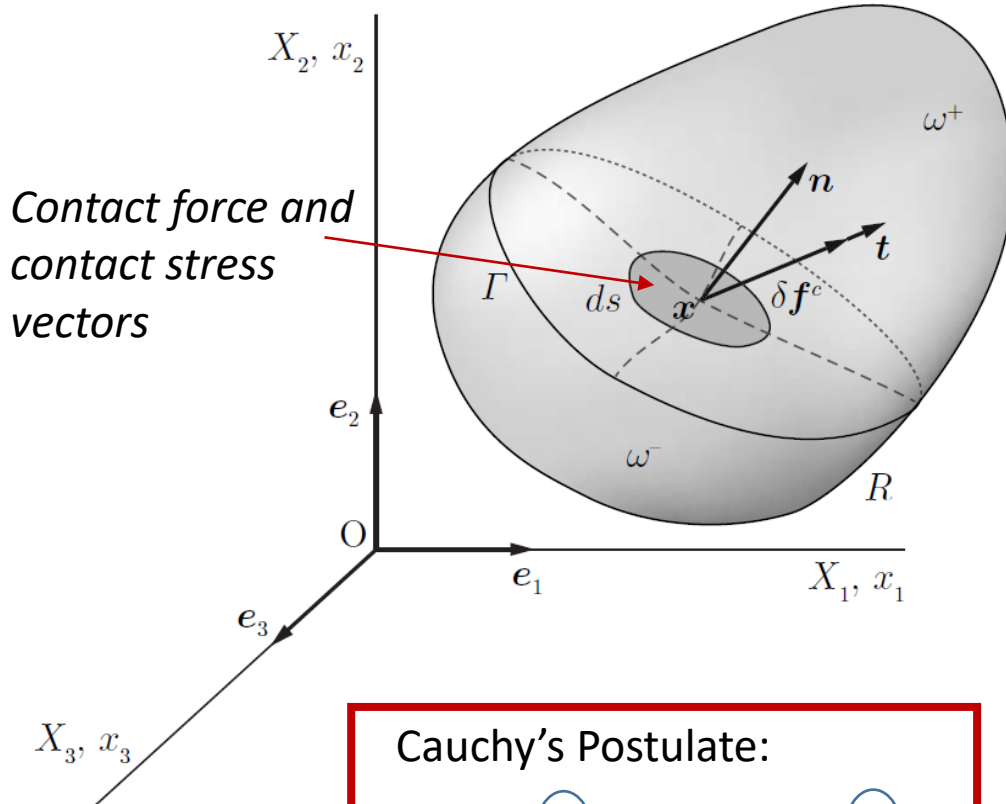
$$\mathbf{f}^b(\omega, t) = \mathbf{F}^b(\Omega, t)$$

or

$$\int_{\omega} \rho(\mathbf{x}, t) \mathbf{b}(\mathbf{x}, t) dv - \int_{\Omega} P_0(\mathbf{X}) \mathbf{B}(\mathbf{X}, t) dV = 0$$

# Continuum mechanics review: Dynamics

## CONTACT (SURFACE) FORCES



Cauchy's Postulate:

$$t(x, t, \Gamma) = t(x, t, n)$$

The action of  $\Pi^+$  on  $\Pi^-$  is:

$$f^c(\Gamma, t) = \int_{\Gamma} t(x, t, n) ds$$

**Contact forces** can describe

1: the interaction between two interior parts of a body  $\mathcal{B}$  separated by a surface (i.e., internal cohesive forces).

2: the action of external bodies directly in contact with the boundary of  $\mathcal{B}$

We consider the body in two parts  $\Pi^-$ ;  $\Pi^+$  with volumes:

$$\omega^- \subset \mathcal{R} \text{ and } \omega^+ \subset \mathcal{R}$$

The two parts are separated by a boundary  $\Gamma$ .

At a time  $t$ , the action of  $\Pi^+$  on  $\Pi^-$ , accros  $ds(x)$  is  $\delta f^c(x, t, \Gamma)$ .

$$t(x, t, \Gamma) = \lim_{\delta s \rightarrow 0} \frac{\delta f^c(x, t, \Gamma)}{\delta s(x)}$$

The limit, when it exists, is called **spatial stress vector**, or **surface stress vector**:

# Continuum mechanics review: Dynamics

## CONSERVATION OF MOMENTUM

In physics, the momentum of a particle with mass  $m$  is defined by:

$$\bar{\mathbf{m}} = m\mathbf{v} \quad \bar{m}_i = mv_i$$

For a part  $\Pi$  of a body  $\mathcal{B}$  initially at  $\mathcal{R}$  and currently at  $\mathcal{R}_t$ , the momentum is:

$$\bar{\mathbf{m}}(\omega, t) = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{v}(\mathbf{x}, t) dv$$

$$\bar{m}_i(\omega, t) = \int_{\omega} \rho(\mathbf{x}, t) v_i(\mathbf{x}, t) dv$$

To study its conservation, we need the material derivative expressed as follows:

$$\begin{aligned} \frac{D\bar{m}_i(\omega, t)}{Dt} &= \frac{d}{dt} \int_{\omega} \rho v_i dv = \int_{\omega} \left( \frac{D(\rho v_i)}{Dt} + \rho v_i \frac{\partial v_m}{\partial x_m} \right) dv \\ &= \int_{\omega} \left( \frac{D\rho}{Dt} v_i + \rho \frac{Dv_i}{Dt} + \rho v_i \frac{\partial v_m}{\partial x_m} \right) dv \\ &= \int_{\omega} \left( \rho \frac{Dv_i}{Dt} + v_i \left( \frac{D\rho}{Dt} + \rho \frac{\partial v_m}{\partial x_m} \right) \right) dv \\ &= \int_{\omega} \rho a_i dv. \end{aligned}$$

Local form of the mass conservation equal to 0

$$\begin{aligned} \frac{D\bar{\mathbf{m}}(\omega, t)}{Dt} &= \int_{\omega} \rho(\mathbf{x}, t) \frac{D\mathbf{v}(\mathbf{x}, t)}{Dt} dv \\ &= \int_{\omega} \rho(\mathbf{x}, t) \mathbf{a}(\mathbf{x}, t) dv \end{aligned}$$

in vector form

# Continuum mechanics review: Dynamics

## CONSERVATION OF ANGULAR MOMENTUM

In physics, the angular momentum of a particle with mass  $m$  is defined by:

$$\widehat{\mathbf{m}} = m\mathbf{x} \times \mathbf{v} \quad \widehat{m}_i = m\varepsilon_{ijk}x_jv_k$$

For a part  $\Pi$  of a body  $\mathcal{B}$  initially at  $\mathcal{R}$  and currently at  $\mathcal{R}_t$ , the momentum is:

$$\widehat{\mathbf{m}}(\omega, t) = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{v}(\mathbf{x}, t) dv$$

$$\widehat{m}_i(\omega, t) = \int_{\omega} \rho(\mathbf{x}, t) \varepsilon_{ijk} x_j v_k(\mathbf{x}, t) dv$$

$$\begin{aligned} \frac{D\widehat{\mathbf{m}}(\omega, t)}{Dt} &= \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \frac{D\mathbf{v}(\mathbf{x}, t)}{Dt} dv \\ &= \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{a}(\mathbf{x}, t) dv, \end{aligned}$$

To study its conservation, we need the material derivative expressed as follows:

$$\begin{aligned} \frac{D\widehat{m}_i(\omega, t)}{Dt} &= \frac{d}{dt} \int_{\omega} \rho \varepsilon_{ijk} x_j v_k dv \\ &= \int_{\omega} \left( \frac{D(\rho \varepsilon_{ijk} x_j v_k)}{Dt} + \rho \varepsilon_{ijk} x_j v_k \frac{\partial v_m}{\partial x_m} \right) dv \\ &= \int_{\omega} \left( \frac{D\rho}{Dt} \varepsilon_{ijk} x_j v_k + \rho \varepsilon_{ijk} \left( \frac{Dx_j}{Dt} v_k + x_j \frac{Dv_k}{Dt} + x_j v_k \frac{\partial v_m}{\partial x_m} \right) \right) dv \\ &= \int_{\omega} \left( \rho \varepsilon_{ijk} x_j \frac{Dv_k}{Dt} + \underbrace{\rho \varepsilon_{ijk} v_j v_k}_{\text{Product of symmetric and antisymmetric tensors equals 0}} + \varepsilon_{ijk} x_j v_k \left( \underbrace{\frac{D\rho}{Dt} + \rho \frac{\partial v_m}{\partial x_m}}_{\text{Local form of the mass Conservation is to equal 0}} \right) \right) dv \\ &= \int_{\omega} \rho \varepsilon_{ijk} x_j a_k dv, \end{aligned}$$

Product of symmetric and antisymmetric tensors equals 0

Local form of the mass Conservation is to equal 0

in vector form

# Continuum mechanics review: Dynamics

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## EULER'S LAWS OF MOTION

### PRINCIPLE OF CONSERVATION OF MOMENTUM

*The rate of change of the momentum of an arbitrary part  $\Pi$  of a body  $\mathcal{B}$  at time  $t$  is equal to the sum of the forces applied to  $\Pi$  at that instant.*

$$\frac{d}{dt} \int_{\omega} \rho(\mathbf{x}, t) \mathbf{v}(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{b}(\mathbf{x}, t) dv + \int_{\partial\omega} \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds .$$

With the conservation of momentum:

$$\frac{d}{dt} \int_{\omega} \rho(\mathbf{x}, t) \mathbf{v}(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{a}(\mathbf{x}, t) dv$$

$$\int_{\omega} \rho(\mathbf{x}, t) \mathbf{a}(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{b}(\mathbf{x}, t) dv + \int_{\partial\omega} \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds$$

It will lead to the equation of motion.

# Continuum mechanics review: Dynamics

## EULER'S LAWS OF MOTION

### PRINCIPLE OF CONSERVATION OF ANGULAR MOMENTUM

*The rate of change of the angular momentum of an arbitrary part  $\Pi$  of a body  $\mathcal{B}$  at time  $t$  is equal to the moment (with respect to the origin) of the forces applied to  $\Pi$  at that instant.*

$$\begin{aligned} \frac{d}{dt} \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{v}(\mathbf{x}, t) dv \\ = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{b}(\mathbf{x}, t) dv + \int_{\partial\omega} \mathbf{x} \times \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds \end{aligned}$$

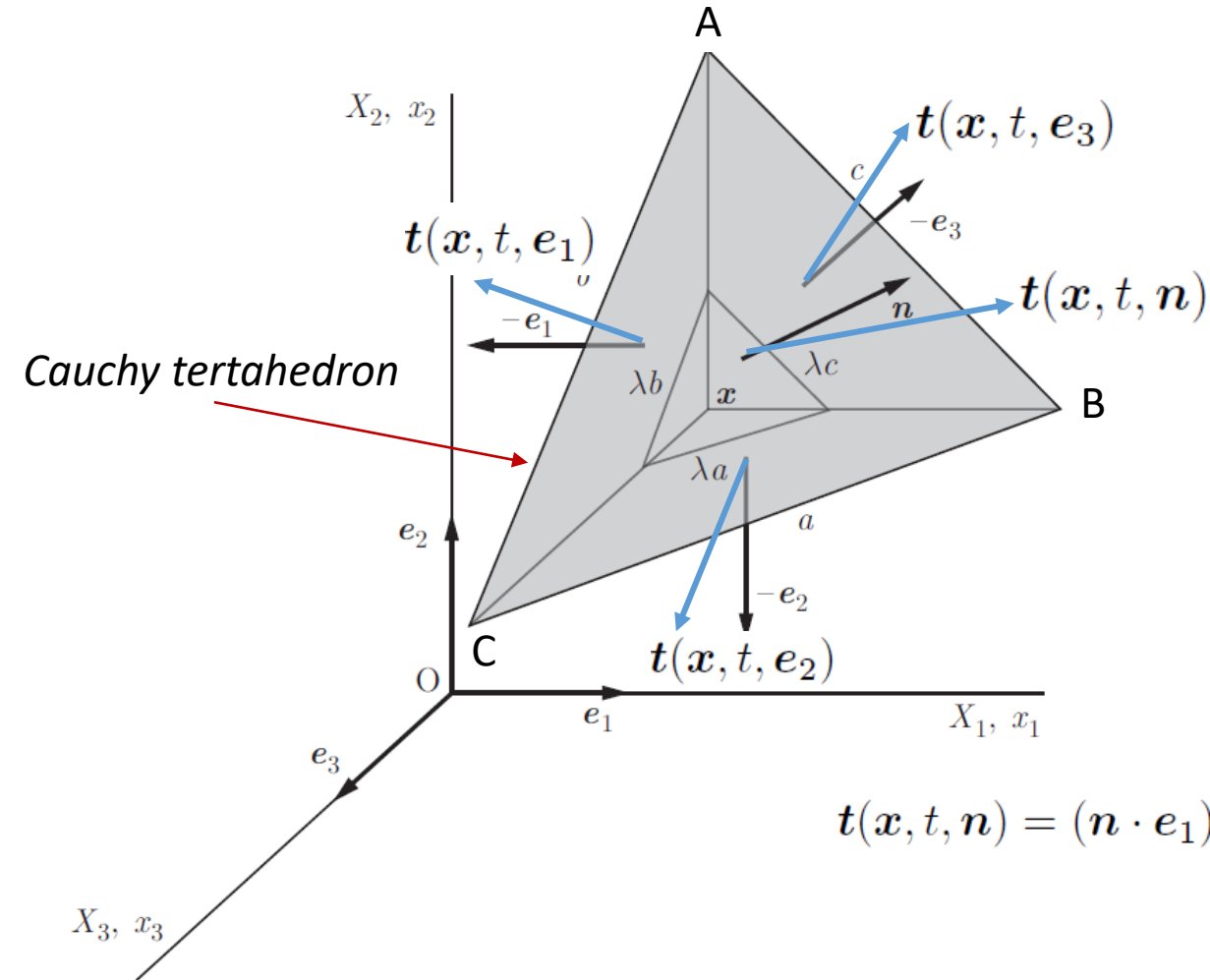
With the conservation of angular momentum:  $\frac{d}{dt} \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{v}(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{a}(\mathbf{x}, t) dv$



$$\begin{aligned} \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{a}(\mathbf{x}, t) dv \\ = \int_{\omega} \rho(\mathbf{x}, t) \mathbf{x} \times \mathbf{b}(\mathbf{x}, t) dv + \int_{\partial\omega} \mathbf{x} \times \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds \end{aligned}$$

It will lead to the symmetry of the stress tensor.

# Continuum mechanics review: Dynamics



## Cauchy's Theorem

If the stress vector  $t(x, t, n)$  is continuous with respect to  $x$  and if  $\rho(x, t)b(x, t)$  and  $\rho(x, t)a(x, t)$  are bounded, the principle of conservation of momentum implies that there exists a 2<sup>nd</sup> order stress tensor  $\sigma(x, t)$ :

$$t_i(x, t, n) = \sigma_{ij}(x, t)n_j$$

We apply the principle of conservation of momentum to the tetrahedron shown on the left to obtain the stress vector on the ABC side in terms of the stress vectors on the three sides of the tetrahedron as follows:

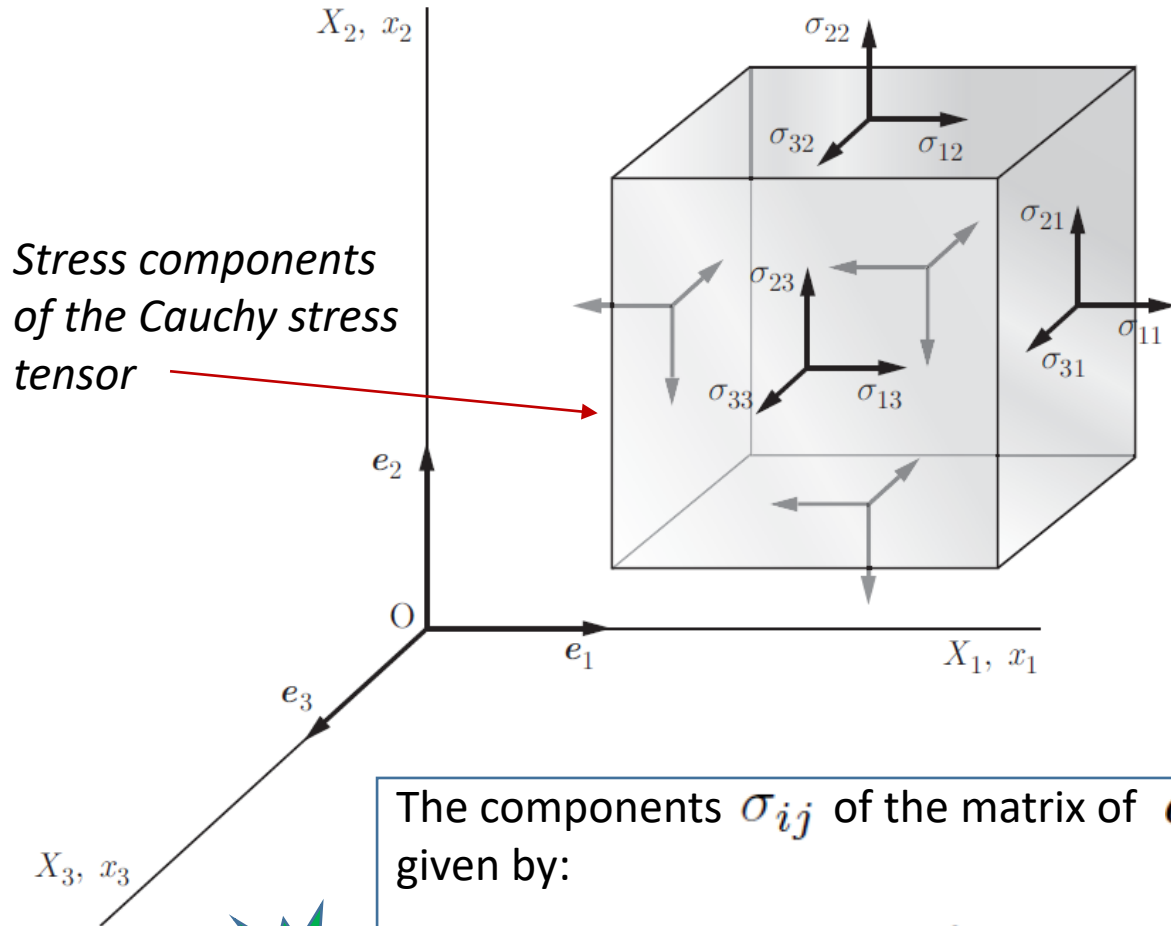
$$t(x, t, n) = (n \cdot e_1)t(x, t, e_1) + (n \cdot e_2)t(x, t, e_2) + (n \cdot e_3)t(x, t, e_3)$$

$$(a \otimes b)v = (b \cdot v)a = a(b \cdot v)$$

$$t(x, t, n) = (t(x, t, e_1) \otimes e_1 + t(x, t, e_2) \otimes e_2 + t(x, t, e_3) \otimes e_3)n$$

$$\sigma(x, t) = t(x, t, e_1) \otimes e_1 + t(x, t, e_2) \otimes e_2 + t(x, t, e_3) \otimes e_3$$

# Continuum mechanics review: Dynamics



## Consequences of the Cauchy's Theorem

The theorem expresses the linear dependence of  $t_i(\mathbf{x}, t, \mathbf{n})$  with respect to the unit normal:

$$t_i(\mathbf{x}, t, \mathbf{n}) = \sigma_{ij}(\mathbf{x}, t) n_j$$

- 1: when the stress tensor  $\sigma(\mathbf{x}, t)$  is known, the stress vector acting at  $\mathbf{x}$  on any surface with outgoing unit normal  $\mathbf{n}$  is completely determined.
- 2: the state of stress at  $\mathbf{x}$  (at time  $t$ ) is characterized by the stress tensor  $\sigma(\mathbf{x}, t)$ .

The components  $\sigma_{ij}$  of the matrix of  $\sigma$  with respect to the basis  $\{e_1, e_2, e_3\}$  are given by:

$\sigma_{ij} = e_i \cdot \sigma e_j = e_i \cdot t_{e_j} \rightarrow \sigma_{ij}$  is the component of the stress vector  $t_{e_j}$  in the direction  $i$  acting on a spatial surface element whose unit normal is aligned in the direction of  $e_j$ .

direction      surface

# Continuum mechanics review: Dynamics

## PRINCIPLE OF CONSERVATION OF MOMENTUM

### Theorem

Suppose that the stress tensor  $\boldsymbol{\sigma}(\mathbf{x}, t)$  is continuously differentiable with respect to  $\mathbf{x}$ , and that  $\rho(\mathbf{x}, t)\mathbf{b}(\mathbf{x}, t)$  and  $\rho(\mathbf{x}, t)\mathbf{a}(\mathbf{x}, t)$  are continuous at  $\mathbf{x}$ . The principle of conservation of momentum:

$$\int_{\omega} \rho(\mathbf{x}, t)\mathbf{a}(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t)\mathbf{b}(\mathbf{x}, t) dv + \int_{\partial\omega} \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds$$

is satisfied if and only if, for an arbitrary point  $\mathbf{x}$  of  $\mathcal{R}_t$ .

$$\text{div}\sigma_{ij}(\mathbf{x}, t) + \rho(\mathbf{x}, t)\mathbf{b}(\mathbf{x}, t) = \rho(\mathbf{x}, t)\mathbf{a}(\mathbf{x}, t) \quad \text{or} \quad \sigma_{ij,j} + \rho b_i = \rho a_i$$

$$t_i(\mathbf{x}, t, \mathbf{n}) = \sigma_{ij}(\mathbf{x}, t)n_j$$

$$\int_{\omega} \rho(\mathbf{x}, t)a_i(\mathbf{x}, t) dv = \int_{\omega} \rho(\mathbf{x}, t)b_i(\mathbf{x}, t) dv + \int_{\partial\omega} \sigma_{ij}(\mathbf{x}, t)n_j ds$$

$$\int_{\omega} (\rho(\mathbf{x}, t)a_i(\mathbf{x}, t) - \rho(\mathbf{x}, t)b_i(\mathbf{x}, t) - \sigma_{ij,j}(\mathbf{x}, t)) dv = 0$$

# Continuum mechanics review: Dynamics

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## PRINCIPLE OF CONSERVATION OF ANGULAR MOMENTUM

### Theorem

Suppose that the stress tensor  $\boldsymbol{\sigma}(\boldsymbol{x}, t)$  is continuously differentiable with respect to  $\boldsymbol{x}$ , and that  $\rho(\boldsymbol{x}, t)\boldsymbol{b}(\boldsymbol{x}, t)$  and  $\rho(\boldsymbol{x}, t)\boldsymbol{a}(\boldsymbol{x}, t)$  are continuous at  $\boldsymbol{x}$ . The principle of conservation of angular momentum:

$$\begin{aligned} \int_{\omega} \rho(\boldsymbol{x}, t) \boldsymbol{x} \times \boldsymbol{a}(\boldsymbol{x}, t) dv \\ = \int_{\omega} \rho(\boldsymbol{x}, t) \boldsymbol{x} \times \boldsymbol{b}(\boldsymbol{x}, t) dv + \int_{\partial\omega} \boldsymbol{x} \times \boldsymbol{t}(\boldsymbol{x}, t, \boldsymbol{n}) ds \end{aligned}$$

implies the symmetry of the stress tensor, i. e.  $\boldsymbol{\sigma}^T = \boldsymbol{\sigma} \quad ; \quad \sigma_{ij} = \sigma_{ji}$


$$t_i(\boldsymbol{x}, t, \boldsymbol{n}) = \sigma_{ij}(\boldsymbol{x}, t) n_j$$



$$\begin{aligned} \int_{\omega} \rho(\boldsymbol{x}, t) \varepsilon_{ijk} x_j a_k(\boldsymbol{x}, t) dv \\ = \int_{\omega} \rho(\boldsymbol{x}, t) \varepsilon_{ijk} x_j b_k(\boldsymbol{x}, t) dv + \int_{\partial\omega} \varepsilon_{ijk} x_j \sigma_{km}(\boldsymbol{x}, t) n_m ds \end{aligned}$$

# Continuum mechanics review: Dynamics

## PRINCIPLE OF CONSERVATION OF ANGULAR MOMENTUM

Apply the divergence theorem to the last term of equation:

$$\begin{aligned} & \int_{\omega} \rho(\mathbf{x}, t) \varepsilon_{ijk} x_j a_k(\mathbf{x}, t) dv \\ &= \int_{\omega} \rho(\mathbf{x}, t) \varepsilon_{ijk} x_j b_k(\mathbf{x}, t) dv + \int_{\partial\omega} \varepsilon_{ijk} x_j \sigma_{km}(\mathbf{x}, t) n_m ds \end{aligned}$$

$$\int_{\partial\omega} \varepsilon_{ijk} x_j \sigma_{km}(\mathbf{x}, t) n_m ds$$

$$= \int_{\omega} \varepsilon_{ijk} (x_{j,m} \sigma_{km}(\mathbf{x}, t) + x_j \sigma_{km,m}(\mathbf{x}, t)) dv$$

$$= \int_{\omega} \varepsilon_{ijk} (\sigma_{kj}(\mathbf{x}, t) + x_j \sigma_{km,m}(\mathbf{x}, t)) dv .$$



$$\begin{aligned} & \int_{\omega} \varepsilon_{ijk} x_j \left( \overbrace{\rho(\mathbf{x}, t) a_k(\mathbf{x}, t) - \rho(\mathbf{x}, t) b_k(\mathbf{x}, t) - \sigma_{km,m}(\mathbf{x}, t)}^{\text{zero (equations of motion)}} \right) dv \\ &= \int_{\omega} \varepsilon_{ijk} \sigma_{kj}(\mathbf{x}, t) dv . \end{aligned}$$

$$\sigma_{jk} = \sigma_{kj}$$



$$\begin{aligned} \varepsilon_{ijk} \sigma_{kj} &= \frac{1}{2} \varepsilon_{ijk} (\sigma_{kj} + \sigma_{jk}) - \frac{1}{2} \varepsilon_{ijk} (\sigma_{jk} - \sigma_{kj}) \\ &= -\frac{1}{2} \varepsilon_{ijk} (\sigma_{jk} - \sigma_{kj}) = 0 \end{aligned}$$



$$\int_{\omega} \varepsilon_{ijk} \sigma_{kj}(\mathbf{x}, t) dv = 0$$



# Continuum mechanics review: Dynamics

## Properties of the Stress Tensor

The stress vector is given by the Cauchy's formula:

$$\mathbf{t} = \boldsymbol{\sigma} \mathbf{n} \quad \text{or} \quad t_i = \sigma_{ij} n_j$$

When the stress vector acts along in the direction on the vector normal to the surface:

$$\boldsymbol{\sigma} \mathbf{n} = \lambda \mathbf{n} \quad \text{or} \quad \sigma_{ij} n_j = \lambda n_i$$



$$\det(\boldsymbol{\sigma} - \lambda \mathbf{I}) = 0 \quad \text{or} \quad \det(\sigma_{ij} - \lambda \delta_{ij}) = 0$$



$$\lambda^3 - I_1(\boldsymbol{\sigma})\lambda^2 + I_2(\boldsymbol{\sigma})\lambda - I_3(\boldsymbol{\sigma}) = 0$$

This is the characteristic equation of an eigenvalue problem.

Here the stress invariants are given by:

$$I_1(\boldsymbol{\sigma}) = \text{tr } \boldsymbol{\sigma} = \sigma_{ii}$$

$$I_2(\boldsymbol{\sigma}) = \frac{1}{2} ((\text{tr } \boldsymbol{\sigma})^2 - \text{tr } \boldsymbol{\sigma}^2) = \frac{1}{2} ((\sigma_{ii})^2 - \sigma_{mn} \sigma_{nm})$$

$$I_3(\boldsymbol{\sigma}) = \det \boldsymbol{\sigma} = \varepsilon_{ijk} \sigma_{i1} \sigma_{j2} \sigma_{k3}$$

The three roots are real (stress tensor is symmetric) and are called principal stresses :

$\sigma_1$ ,  $\sigma_2$  and  $\sigma_3$ .

conventionally ordered as:  $\sigma_1 \geq \sigma_2 \geq \sigma_3$

The corresponding principal eigenvectors are the associated principal directions (or planes)  $\mathbf{n}_i$ .

In terms of the principal stresses the invariants are:

$$I_1(\boldsymbol{\sigma}) = \sigma_1 + \sigma_2 + \sigma_3$$

$$I_2(\boldsymbol{\sigma}) = \sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_3 \sigma_1$$

$$I_3(\boldsymbol{\sigma}) = \sigma_1 \sigma_2 \sigma_3.$$

# Continuum mechanics review: Dynamics

## Properties of the Stress Tensor

The stress vector is given by the Cauchy's formula:

$$\mathbf{t} = \boldsymbol{\sigma} \mathbf{n} \quad \text{or} \quad t_i = \sigma_{ij} n_j$$

The stress vector has a normal  $t_N$  and a tangential  $t_T$  component associated with shear stress:

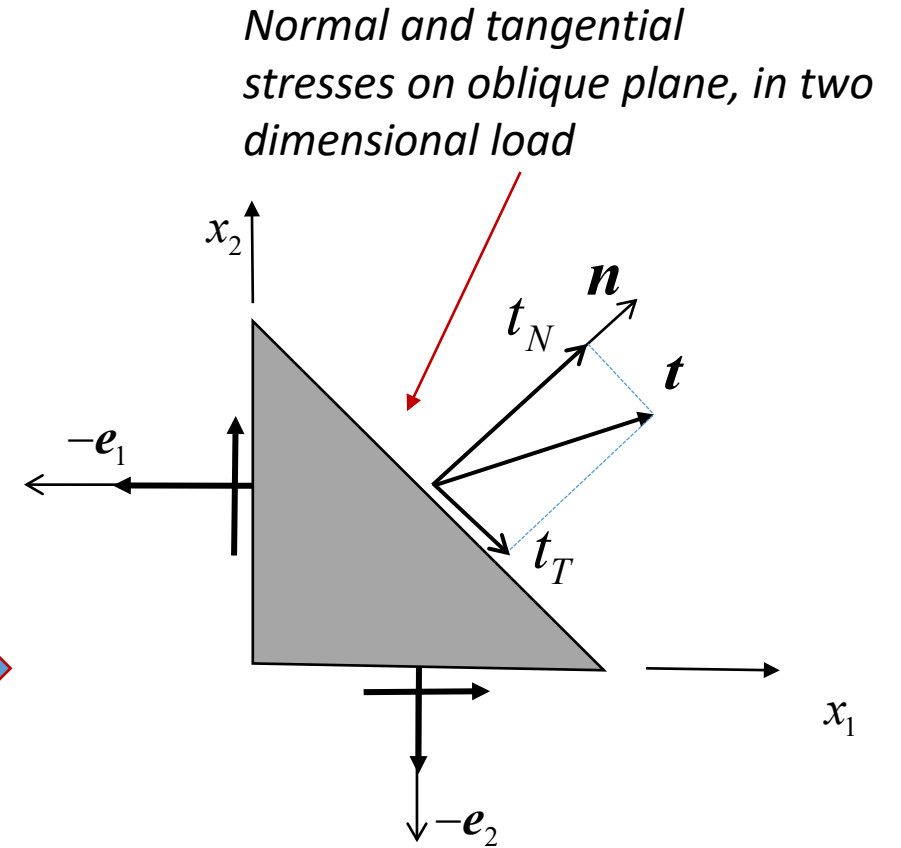
$$t_N = \mathbf{n} \cdot \mathbf{t} = \mathbf{n} \cdot \boldsymbol{\sigma} \mathbf{n} \quad \text{or} \quad t_N = n_i t_i = \sigma_{ij} n_i n_j$$

$$\begin{aligned} t_T &= \|\mathbf{t} - (\mathbf{n} \cdot \mathbf{t}) \mathbf{n}\| = \|(\mathbf{I} - \mathbf{n} \otimes \mathbf{n}) \mathbf{t}\| \\ &= (t_i t_i - t_N^2)^{1/2} \end{aligned}$$

When the stress vector acts along the direction of the vector normal to the surface:

$$\boldsymbol{\sigma} \mathbf{n} = \lambda \mathbf{n} \quad \text{or} \quad \sigma_{ij} n_j = \lambda n_i$$

$$\Rightarrow \lambda^3 - I_1(\boldsymbol{\sigma}) \lambda^2 + I_2(\boldsymbol{\sigma}) \lambda - I_3(\boldsymbol{\sigma}) = 0$$



# Continuum mechanics review: Dynamics

## Stress Vector at a plane $\mathbf{n}$

The stress vector is given by the Cauchy's formula:

$$\mathbf{t} = \boldsymbol{\sigma} \mathbf{n} \quad \text{or} \quad t_i = \sigma_{ij} n_j$$

The stress vector has a normal  $t_N$  and a tangential  $t_T$  component associated with shear stress:

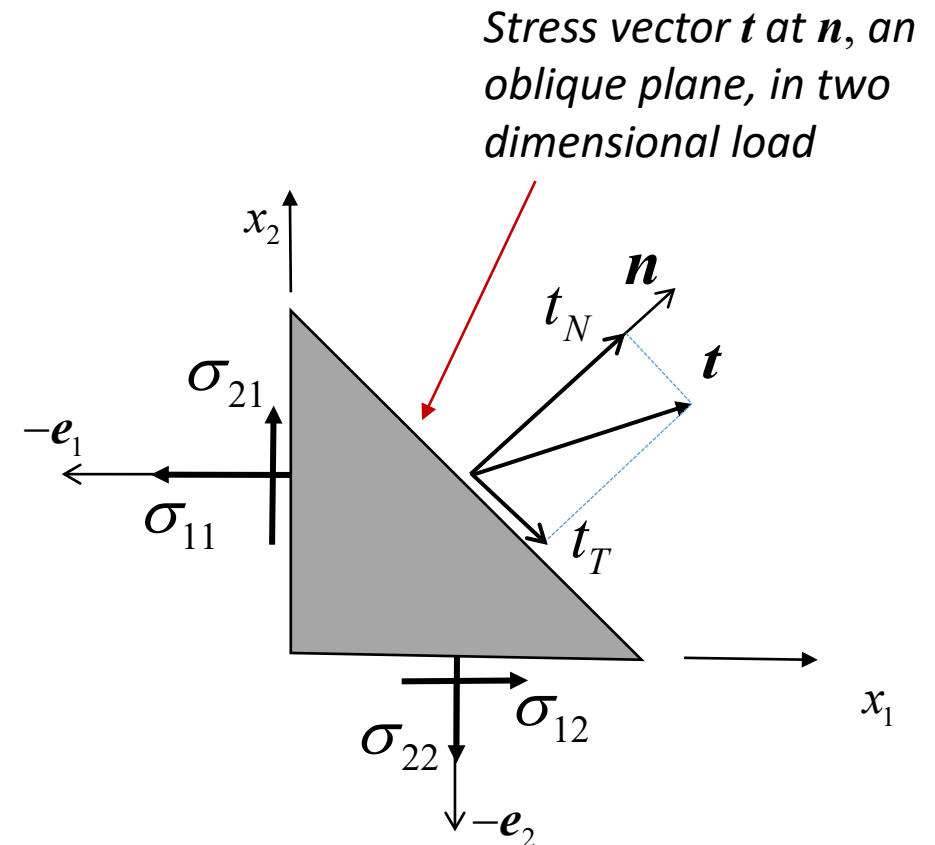
$$t_N = \mathbf{n} \cdot \mathbf{t} = \mathbf{n} \cdot \boldsymbol{\sigma} \mathbf{n} \quad \text{or} \quad t_N = n_i t_i = \sigma_{ij} n_i n_j$$

$$\begin{aligned} t_T &= \|\mathbf{t} - (\mathbf{n} \cdot \mathbf{t}) \mathbf{n}\| = \|(\mathbf{I} - \mathbf{n} \otimes \mathbf{n}) \mathbf{t}\| \\ &= (t_i t_i - t_N^2)^{1/2} \end{aligned}$$

The expression:

$$t_i = \sigma_{ij} n_j$$

is used to calculate the stress vector  $\mathbf{t}$  at any given plane with normal  $\mathbf{n}$  passing by a point  $\mathbf{x}$



Octahedral plane: It is a special plane: Its unit normal vector has equal components with respect to all three principal directions

$$\mathbf{m} = (1/\sqrt{3})\mathbf{n}_1 + (1/\sqrt{3})\mathbf{n}_2 + (1/\sqrt{3})\mathbf{n}_3$$

# Continuum mechanics review: Dynamics

## Transformation of the stress tensor

The components  $\sigma_{ij}$  of the stress tensor  $\boldsymbol{\sigma}$  form a 3x3 matrix given by (relative to  $\{e_1, e_2, e_3\}$ ):

$$\sigma_{ij} = e_i \cdot \boldsymbol{\sigma} e_j$$

Consider another orthogonal basis  $\{e'_1, e'_2, e'_3\}$  obtained by rotation of  $\{e_1, e_2, e_3\}$ :

$$e'_i = c_{ij} e_j \quad (i = 1, 2, 3)$$

$$\begin{aligned} \Rightarrow \sigma'_{ij} &= e'_i \cdot \boldsymbol{\sigma} e'_j \\ &= c_{im} c_{jn} e_m \cdot \boldsymbol{\sigma} e_n = c_{im} c_{jn} \sigma_{mn} \end{aligned}$$

In matrix form the transformation is:  $[\sigma'] = [C][\sigma][C]^T$

The elements of  $[C]$  are the direction cosines of the  $\{e'_1, e'_2, e'_3\}$  with respect to  $\{e_1, e_2, e_3\}$  and

$$[C][C]^T = [C][C]^{-1} = [I]$$

## Deviatoric stress tensor

It is often useful to decompose  $\boldsymbol{\sigma}$  in two components as follows:

$$\boldsymbol{\sigma} = \boldsymbol{s} + \sigma_0 \boldsymbol{I} \quad \text{or} \quad \sigma_{ij} = s_{ij} + \sigma_0 \delta_{ij}$$

where  $\boldsymbol{s} = \boldsymbol{\sigma} - \sigma_0 \boldsymbol{I}$

$$\sigma_0 = \frac{1}{3} I_1(\boldsymbol{\sigma}) = \frac{1}{3} \sigma_{kk}$$

Tensor  $\boldsymbol{s}$  is called deviatoric stress tensor associated with  $\boldsymbol{\sigma}$  and the property:

$$\text{tr } \boldsymbol{s} = s_{ii} = 0$$

# Stress Analysis: Principal stresses

## EXAMPLE 1.8

Find the principal values (eigenvalues) and the corresponding unit vectors (eigenvectors) of the symmetric tensor with matrix

$$[L] = \begin{pmatrix} 2 & 1 & -2 \\ 1 & 4 & -3 \\ -2 & -3 & -2 \end{pmatrix}. \quad (1.122)$$

Using the expressions below, we determine the invariants, characteristic equation and its roots:



$$I_1(\mathbf{L}) = L_{ii} = \text{tr } \mathbf{L}$$

$$\begin{aligned} I_2(\mathbf{L}) &= \begin{vmatrix} L_{11} & L_{12} \\ L_{21} & L_{22} \end{vmatrix} + \begin{vmatrix} L_{22} & L_{23} \\ L_{32} & L_{33} \end{vmatrix} + \begin{vmatrix} L_{11} & L_{13} \\ L_{31} & L_{33} \end{vmatrix} \\ &= \frac{1}{2} (L_{ii}L_{jj} - L_{ij}L_{ji}) \\ &= \frac{1}{2} ((\text{tr } \mathbf{L})^2 - \text{tr } (\mathbf{L}\mathbf{L})) = \frac{1}{2} ((\text{tr } \mathbf{L})^2 - \text{tr } (\mathbf{L}^2)) \end{aligned}$$

$$I_3(\mathbf{L}) = \varepsilon_{ijk} L_{i1} L_{j2} L_{k3} = \det \mathbf{L}.$$

$$I_1(\mathbf{L}) = 4, \quad I_2(\mathbf{L}) = -18, \quad I_3(\mathbf{L}) = -36,$$



$$\lambda^3 - 4\lambda^2 - 18\lambda + 36 = 0.$$



$$\lambda_1 = 6, \quad \lambda_2 = 1.65, \quad \lambda_3 = -3.65.$$

For the principal directions see the rest of the example in the book (Botsis & Deville)

# Stress Analysis: Stress vector on a plane

We consider the tensor of stresses,

$$[\sigma] = \begin{pmatrix} 2 & 1 & -2 \\ 1 & 4 & -3 \\ -2 & -3 & -2 \end{pmatrix}$$

To calculate the stress vector at a point belonging to the plane with outward normal given by:

$$\mathbf{n} = 2\mathbf{e}_1/3 + 2\mathbf{e}_2/3 - \mathbf{e}_3/3:$$

we use the Cauchy expression

$$t_i = \sigma_{ij}n_j$$



$$\begin{pmatrix} t_1 \\ t_2 \\ t_3 \end{pmatrix} = \begin{pmatrix} 2 & 1 & -2 \\ 1 & 4 & -3 \\ -2 & -3 & -2 \end{pmatrix} \begin{pmatrix} 2/3 \\ 2/3 \\ -1/3 \end{pmatrix} = \frac{1}{3} \begin{pmatrix} 8 \\ 13 \\ -8 \end{pmatrix}$$

We can also calculate the normal and tangential components on the plane using the expressions



$$t_N = n_i t_i = 5.55$$

$$t_T = (t_i t_i - t_N^2)^{1/2} = 4.56$$



# Stress Analysis: Octahedral plane

We take next the stress tensor with its principal values

$$\begin{pmatrix} 6 & 0 & 0 \\ 0 & 1.65 & 0 \\ 0 & 0 & -3.65 \end{pmatrix}$$

and consider a coordinate system defined by the principal directions and the following plane with normal:

$$\mathbf{m} = (1/\sqrt{3})\mathbf{n}_1 + (1/\sqrt{3})\mathbf{n}_2 + (1/\sqrt{3})\mathbf{n}_3.$$

We calculate the stress vector on that plane using

$$t_i = \sigma_{ij}n_j$$



$$\begin{pmatrix} t_1 \\ t_2 \\ t_3 \end{pmatrix} = \begin{pmatrix} 6 & 0 & 0 \\ 0 & 1.65 & 0 \\ 0 & 0 & -3.65 \end{pmatrix} \begin{pmatrix} 1/\sqrt{3} \\ 1/\sqrt{3} \\ 1/\sqrt{3} \end{pmatrix} = \frac{1}{\sqrt{3}} \begin{pmatrix} 6 \\ 1.65 \\ -3.65 \end{pmatrix}$$



The plane so defined is called  
**OCTAHEDRAL PLANE**

$$t_T = (t_i t_i - t_N^2)^{1/2} = 0.94.$$

$$t_N = n_i t_i = 1.33$$

On that plane the normal and tangential components are

# Stress Analysis: Octahedral stresses

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On the octahedral plane we have two stress components (octahedral stresses):

$$t_N = I_1(\boldsymbol{\sigma})/3 \quad \leftarrow \text{Octahedral normal}$$
$$t_T = \frac{1}{3} \sqrt{2I_1^2(\boldsymbol{\sigma}) - 6I_2(\boldsymbol{\sigma})} \quad \leftarrow \text{Octahedral shear}$$

$$t_T = \frac{1}{3} [(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2]^{1/2} = \sqrt{\frac{2}{3} I_2(\mathbf{s})}$$

Note that the latter expression is proportional to the von Mises stress:

$$\sigma_e = \left[ \frac{1}{2} ((\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2) \right]^{1/2} = \sqrt{\frac{1}{3} I_2(\mathbf{s})}$$

# Continuum mechanics review: Dynamics

## Equilibrium equations for a continuous medium

We set the acceleration zero in the equations of motion:

$$\operatorname{div} \sigma_{ij}(\mathbf{x}, t) + \rho(\mathbf{x}, t) b(\mathbf{x}, t) = \rho(\mathbf{x}, t) a(\mathbf{x}, t)$$

to obtain the equilibrium eqs:

$$\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} + \frac{\partial \sigma_{13}}{\partial x_3} + \rho b_1 = 0$$

$$\frac{\partial \sigma_{21}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} + \frac{\partial \sigma_{23}}{\partial x_3} + \rho b_2 = 0$$

$$\frac{\partial \sigma_{31}}{\partial x_1} + \frac{\partial \sigma_{32}}{\partial x_2} + \frac{\partial \sigma_{33}}{\partial x_3} + \rho b_3 = 0$$

or 
$$\frac{\partial \sigma_{ij}}{\partial x_j} + \rho b_i = 0$$

( $\rho$  is the mass density and  $b_i$  the volume force density)

## Uniform tension or compression

Tension is applied in direction 1.  $\boldsymbol{\sigma}$  is given by:

$$\boldsymbol{\sigma} = \sigma \mathbf{n}_1 \otimes \mathbf{n}_1 \quad \text{or} \quad [\sigma] = \begin{pmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

( $\sigma$  is a constant)

This state of stress is encountered in a prismatic bar parallel to the direction 1.

More general uniform tension or compression in a direction  $\mathbf{m}$  is expressed as:

$$\boldsymbol{\sigma} = \sigma (\mathbf{m} \otimes \mathbf{m}) \quad \text{or} \quad \sigma_{ij} = \sigma m_i m_j$$

( $\sigma$  is a constant).

# Continuum mechanics review: Dynamics

## Uniform shear

A uniform shear is applied in direction 1 on the planes perpendicular to  $\mathbf{e}_2$ . The stress tensor becomes:

$$\boldsymbol{\sigma} = \tau(\mathbf{e}_1 \otimes \mathbf{e}_2 + \mathbf{e}_2 \otimes \mathbf{e}_1) \text{ where } \tau \geq 0$$

or

$$[\sigma] = \begin{pmatrix} 0 & \tau & 0 \\ \tau & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

The characteristic equation becomes:

$$\lambda(\lambda^2 - \tau^2) = 0$$

With solutions as the principal stresses:

$$\sigma_1 = \tau, \sigma_2 = 0, \sigma_3 = -\tau$$

And corresponding principal directions:

$$\mathbf{n}_1 = (\mathbf{e}_1 + \mathbf{e}_2)/\sqrt{2}, \quad \mathbf{n}_2 = \mathbf{e}_3$$

$$\mathbf{n}_3 = (\mathbf{e}_1 - \mathbf{e}_2)/\sqrt{2}$$

## Uniform tension or compression

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$$\boldsymbol{\sigma} = \sigma \mathbf{n}_1 \otimes \mathbf{n}_1 \quad \text{or} \quad [\sigma] = \begin{pmatrix} \sigma & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

( $\sigma$  is a constant)

This state of stress is encountered in a prismatic bar parallel to the direction 1.



More general uniform tension or compression in a direction  $\mathbf{m}$  is expressed as:

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# Continuum mechanics review: Dynamics

## Uniform shear

A uniform shear is applied in direction 1 on the planes perpendicular to  $\mathbf{e}_2$ . The stress tensor becomes:

$$\boldsymbol{\sigma} = \tau(\mathbf{e}_1 \otimes \mathbf{e}_2 + \mathbf{e}_2 \otimes \mathbf{e}_1) \text{ where } \tau \geq 0$$

or

$$[\sigma] = \begin{pmatrix} 0 & \tau & 0 \\ \tau & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

The characteristic equation becomes:

$$\lambda(\lambda^2 - \tau^2) = 0$$

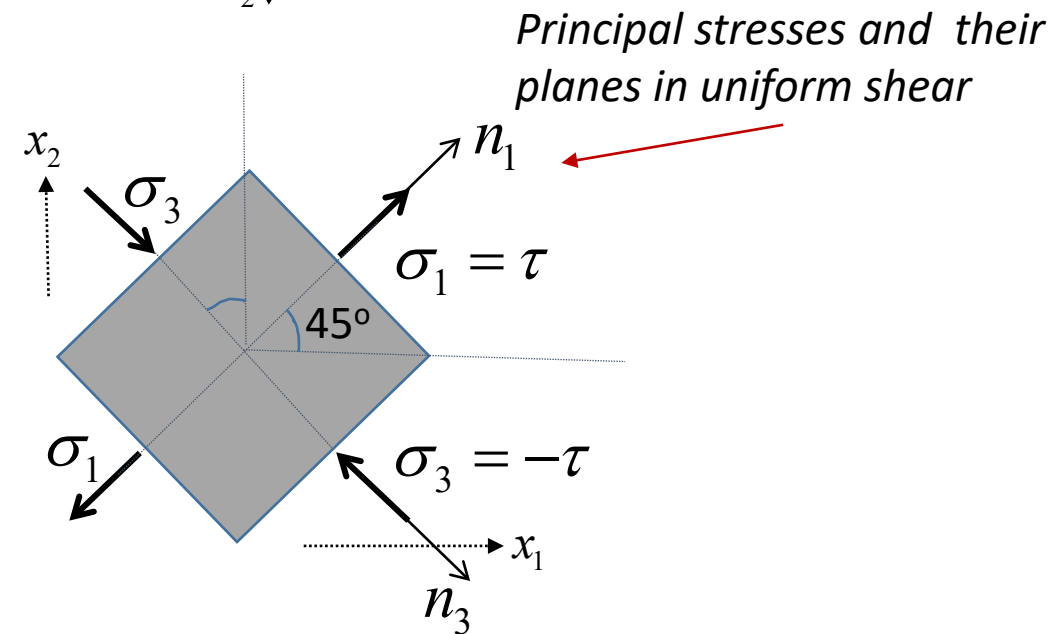
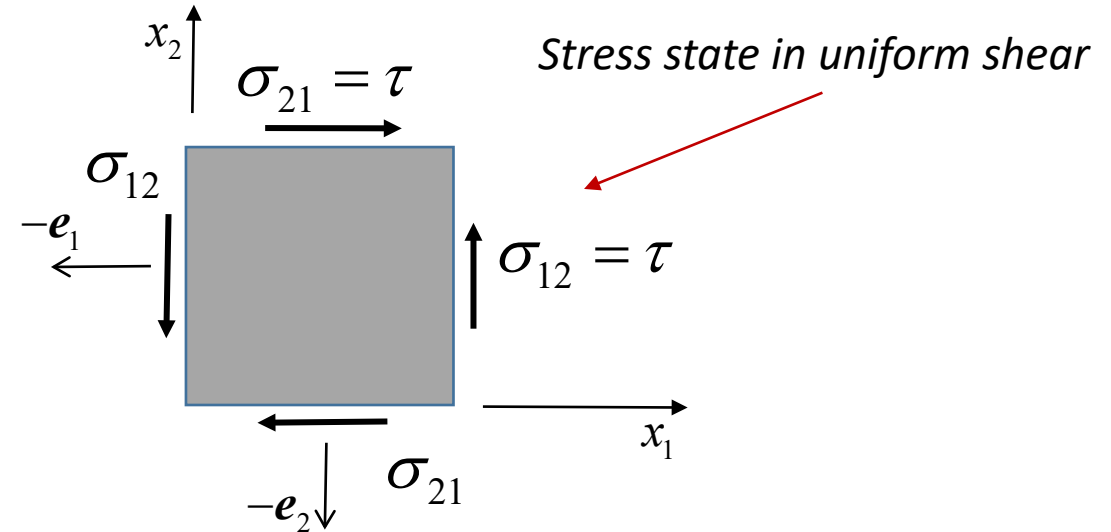
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And corresponding principal directions:

$$\mathbf{n}_1 = (\mathbf{e}_1 + \mathbf{e}_2)/\sqrt{2}, \quad \mathbf{n}_2 = \mathbf{e}_3$$

$$\mathbf{n}_3 = (\mathbf{e}_1 - \mathbf{e}_2)/\sqrt{2}$$



# Continuum mechanics review: Dynamics

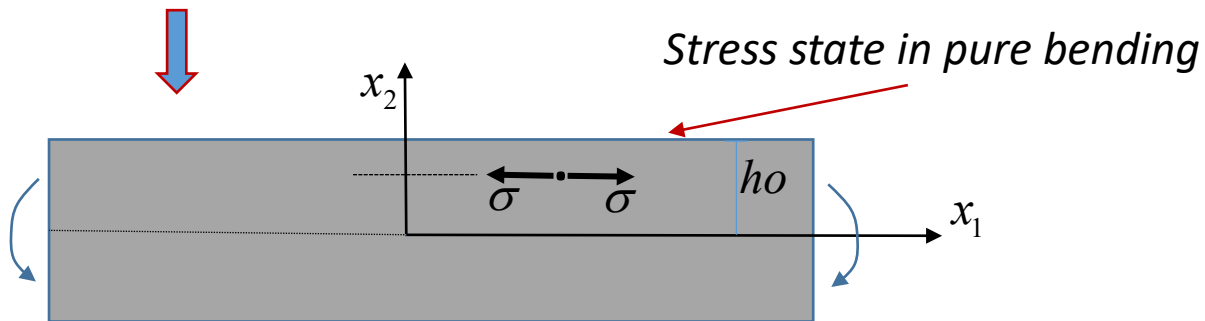
## Pure Bending

Assuming zero body forces, the stress at  $x_2$  is ( $\alpha$  and  $h_0$  are constants):

$$\boldsymbol{\sigma} = \alpha(x_2 - h_0)\mathbf{e}_1 \otimes \mathbf{e}_1$$

or

$$[\sigma] = \begin{pmatrix} \alpha(x_2 - h_0) & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}$$



## Hydrostatic pressure

Here the stress tensor takes the form:

$$\boldsymbol{\sigma} = -p(\mathbf{x})\mathbf{I} \quad \text{or} \quad \sigma_{ij} = -p(\mathbf{x})\delta_{ij}$$

With the equilibrium equations reducing to:

$$-\nabla p + \rho \mathbf{b} = 0 \quad \text{or} \quad -p_{,i} + \rho b_i = 0$$

With the body force zero, a body subjected to a constant hydrostatic pressure is in equilibrium since:

$$-\nabla p = 0$$

# Continuum mechanics review: Dynamics

## Plane stress:

It defined when  $\sigma_{11}$ ,  $\sigma_{22}$ ,  $\sigma_{12} = \sigma_{21}$  may not be zero while the other components are zero. We have:

$$\boldsymbol{\sigma} = \sigma_{11}\mathbf{e}_1 \otimes \mathbf{e}_1 + \sigma_{22}\mathbf{e}_2 \otimes \mathbf{e}_2 + \sigma_{12}(\mathbf{e}_1 \otimes \mathbf{e}_2 + \mathbf{e}_2 \otimes \mathbf{e}_1)$$

Or

$$[\sigma] = \begin{pmatrix} \sigma_{11} & \sigma_{12} & 0 \\ \sigma_{12} & \sigma_{22} & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

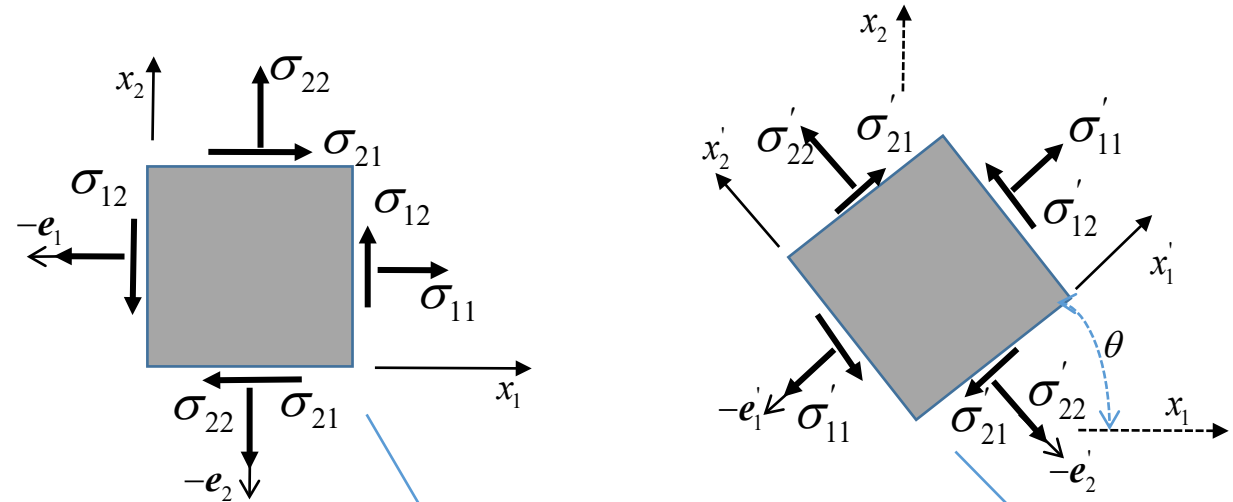
The stresses are only functions of  $x_1$  and  $x_2$ . Assuming zero body forces, the equilibrium equations become:

$$\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} = 0$$

$$\frac{\partial \sigma_{21}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} = 0$$

From  $[\sigma'] = [C][\sigma][C]^T$

## Stress components at a point in plane stress in two coordinate systems



$$\begin{pmatrix} \cos \theta & \sin \theta \\ -\sin \theta & \cos \theta \end{pmatrix} \begin{pmatrix} \sigma_{11} & \sigma_{12} \\ \sigma_{21} & \sigma_{22} \end{pmatrix} \begin{pmatrix} \cos \theta & -\sin \theta \\ \sin \theta & \cos \theta \end{pmatrix} = \begin{pmatrix} \sigma'_{11} & \sigma'_{12} \\ \sigma'_{21} & \sigma'_{22} \end{pmatrix}$$

$(\sigma_{12} = \sigma_{21})$

$$\begin{aligned} \sigma'_{11} &= \sigma_{11} \cos^2 \theta + \sigma_{22} \sin^2 \theta + 2\sigma_{12} \cos \theta \sin \theta \\ \sigma'_{22} &= \sigma_{11} \sin^2 \theta + \sigma_{22} \cos^2 \theta - 2\sigma_{12} \cos \theta \sin \theta \\ \sigma'_{12} &= (\sigma_{22} - \sigma_{11}) \cos \theta \sin \theta + \sigma_{12} (\cos^2 \theta - \sin^2 \theta) \end{aligned}$$

Note that a Mohr's circle for stress analysis is the geometrical solution/intepretation of these equations.

# Continuum mechanics review: Dynamics

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## PIOLA-KIRCHHOFF STRESS TENSORS

The Cauchy stress tensor  $\boldsymbol{\sigma}$  is expressed with respect to the current configuration  $\mathcal{R}$  i.e. it is the real stress.

The principles of momentum and angular momentum are formulated with respect to the current configuration.

Problems in solid mechanics require a formulation with respect to the initial configuration  $\mathcal{R}_0$ .

This is because (a) it is difficult to know the deformed condition of a solid beforehand, (b) it is more convenient to analyze the experimental response of a solid with respect to its undeformed configuration.

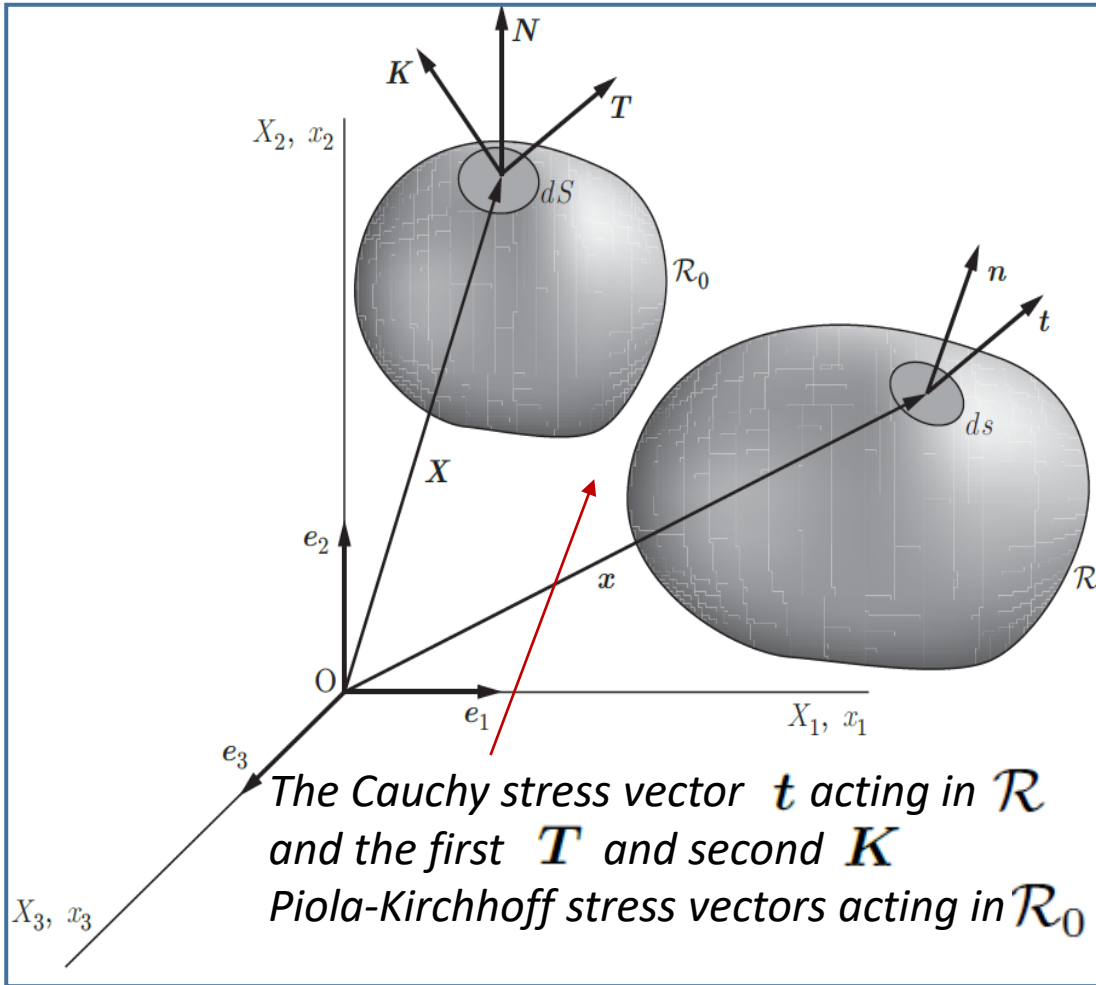
There is not simply a change of variables in the equations of motion and the Cauchy stress components using:

$$\boldsymbol{x} = \boldsymbol{\chi}(\boldsymbol{X}, t)$$

Measurements of stresses in the undeformed configuration have been proposed for the study of problems in solid mechanics.

These are the first and second Piola-Kirchhoff stress tensors.

# Continuum mechanics review: Dynamics



Here  $\mathbf{t}(\mathbf{x}, t, \mathbf{n})$  is the Cauchy stress vector acting on the actual surface element  $\mathbf{n} ds$  at  $\mathbf{x}$ .

To this vector we associate the vector  $\mathbf{T}(\mathbf{X}, t, \mathbf{N})$  Called the first Piola-Kirchhoff stress vector, to the corresponding reference surface element  $\mathbf{N} dS$ , and related to  $\mathbf{t}(\mathbf{x}, t, \mathbf{n})$  as follows:

$$\mathbf{T}(\mathbf{X}, t, \mathbf{N}(\mathbf{X})) dS = \mathbf{t}(\mathbf{x}, t, \mathbf{n}(\mathbf{x}, t)) ds$$

$dS$  and  $ds$  are positive. Thus,  $\mathbf{T}$  and  $\mathbf{t}$  have the same direction but  $\|\mathbf{T}\|$  and  $\|\mathbf{t}\|$  are not generally the same.

Note that the stress vector is not real (often called pseudo-stress).

Using Cauchy's relation  $t_i(\mathbf{x}, t, \mathbf{n}) = \sigma_{ij}(\mathbf{x}, t) n_j$   
and Nanson's formula  $\mathbf{n} ds = J \mathbf{F}^{-T} \mathbf{N} dS$

$$\begin{aligned} \mathbf{T}(\mathbf{X}, t, \mathbf{N}) dS &= \mathbf{t}(\mathbf{x}, t, \mathbf{n}) ds = \boldsymbol{\sigma}(\mathbf{x}, t) \mathbf{n} ds \\ &= J(\mathbf{X}, t) \boldsymbol{\sigma}(\boldsymbol{\chi}(\mathbf{X}, t), t) \mathbf{F}^{-T} \mathbf{N} dS \end{aligned}$$

# Continuum mechanics review: Dynamics

## PIOLA-KIRCHHOFF STRESS TENSORS

$$\begin{aligned} T(X, t, N) dS &= t(x, t, n) ds = \sigma(x, t) n ds \\ &= J(X, t) \sigma(\chi(X, t), t) F^{-T} N dS \end{aligned} \quad \Rightarrow \quad T(X, t, N) = P(X, t) N$$

The First Piola-Kirchhoff stress tensor defined as:

$$P(X, t) = J(X, t) \sigma(\chi(X, t), t) F^{-T}$$

Using the symmetry of the Cauchy's stress tensor  $\sigma = \sigma^T$  it is shown below that:  $P F^T = F P^T$

$$\begin{aligned} P &= J \sigma F^{-T} \Rightarrow P F^T = J \sigma \\ (P F^T)^T &= F P^T = J (\sigma)^T = J \sigma \\ \Rightarrow P F^T &= F P^T \end{aligned}$$

Tensor  $P$  is not symmetric and the principle of conservation of angular momentum is satisfied when this relation is met.

# Continuum mechanics review: Dynamics

## Objectivity of the Tensor $\sigma$

We consider Cauchy's relation  $t_i(\mathbf{x}, t, \mathbf{n}) = \sigma_{ij}(\mathbf{x}, t) n_j$  as seen by two observers  $\mathcal{R}$  and  $\mathcal{R}^*$ .

We assume that vectors  $\mathbf{t}$  and  $\mathbf{n}$  are objective and are transformed as:

$$\mathbf{t}^* = \mathbf{Q}\mathbf{t} \quad ; \quad \mathbf{n}^* = \mathbf{Q}\mathbf{n}$$

From  $\mathbf{t}^* = \sigma^* \mathbf{n}^*$  we have  $\mathbf{Q}\mathbf{t} = \sigma^* \mathbf{Q}\mathbf{n}$

We multiply  $\mathbf{t} = \sigma \mathbf{n}$  by  $\mathbf{Q}$  to obtain:  $\mathbf{Q}\mathbf{t} = \mathbf{Q}\sigma \mathbf{n}$

Comparing the two last results we have:

$$\sigma^* = \mathbf{Q}\sigma \mathbf{Q}^T$$

 the Cauchy's stress tensor is objective.

## Objectivity of the Tensor $\mathbf{P}$

To check the objectivity of  $\mathbf{P}$  we start with

$$\mathbf{P}^* \mathbf{F}^{*T} = J^* \sigma^*$$

and use

$$\mathbf{F}^* = \mathbf{Q}\mathbf{F} \quad ; \quad J^* = J \quad ; \quad \mathbf{P} = J\sigma \mathbf{F}^{-T}$$

$$\mathbf{P}^* (\mathbf{Q}\mathbf{F})^T = J \mathbf{Q}\sigma \mathbf{Q}^T$$

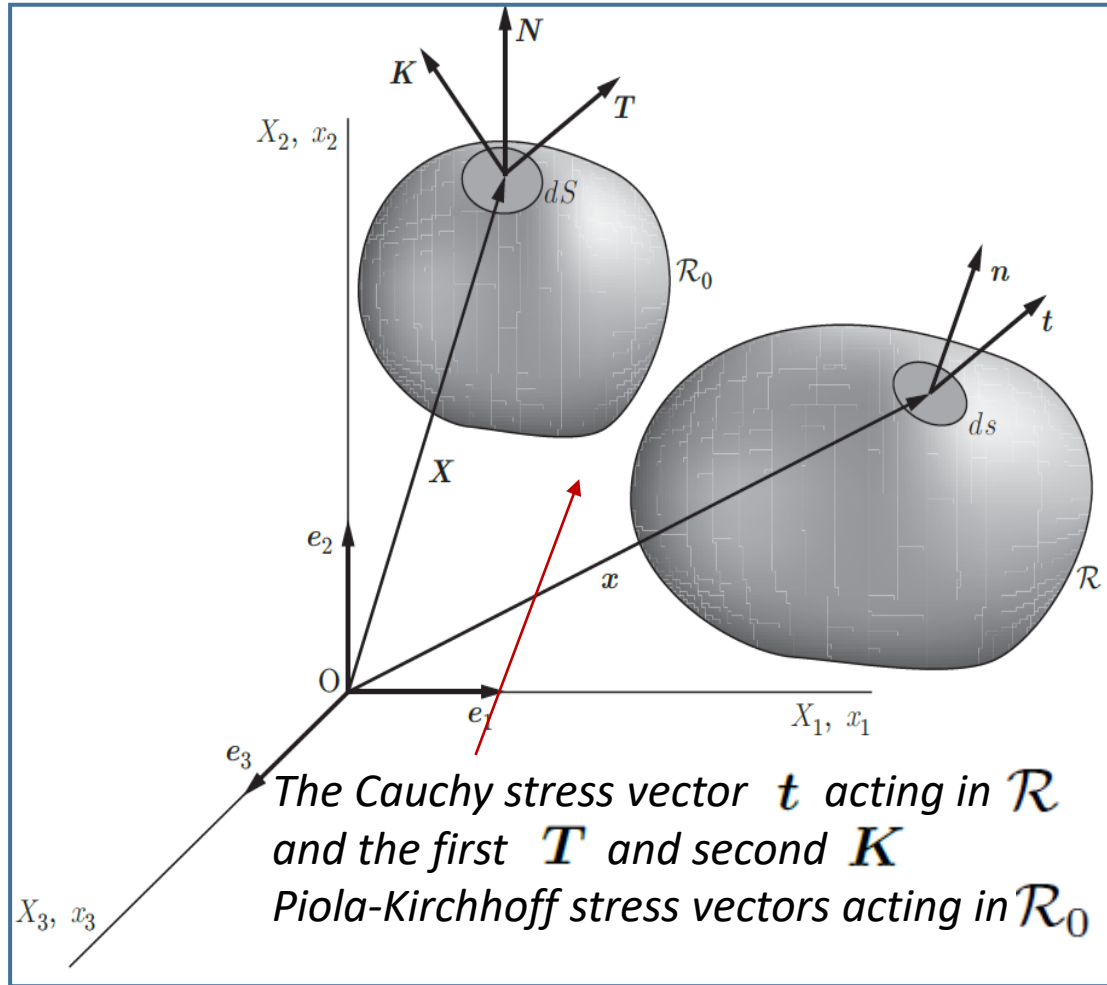
$$\mathbf{P}^* \mathbf{F}^T \mathbf{Q}^T = \mathbf{Q} J \sigma \mathbf{Q}^T = \mathbf{Q} \mathbf{P} \mathbf{F}^T \mathbf{Q}^T$$

$$\mathbf{P}^* = \mathbf{Q} \mathbf{P}.$$



The first Piola-Kirchhoff stress tensor is not objective.

# Continuum mechanics review: Dynamics



The Cauchy stress vector  $\mathbf{t}$  acting in  $\mathcal{R}$  and the first  $\mathbf{T}$  and second  $\mathbf{K}$  Piola-Kirchhoff stress vectors acting in  $\mathcal{R}_0$

Here  $\mathbf{t}(\mathbf{x}, t, \mathbf{n})$  is the Cauchy stress vector acting on the actual surface element  $\mathbf{n} ds$  at  $\mathbf{x}$ .

To this vector we associate the vector called the second Piola-Kirchhoff stress vector, to the corresponding reference surface element  $\mathbf{N} dS$ , and related to  $\mathbf{t}(\mathbf{x}, t, \mathbf{n})$  as follows:

$$\mathbf{K}(\mathbf{X}, t, \mathbf{N}) dS = \mathbf{F}^{-1}(\mathbf{X}, t) \mathbf{t}(\chi(\mathbf{X}, t), t, \mathbf{n}(\mathbf{X}, t)) ds$$

Here  $\mathbf{K}$  expresses the contact force per unit reference surface transformed by  $\mathbf{F}^{-1}$ : Expressing

$$\mathbf{K}(\mathbf{X}, t, \mathbf{N}) = \mathbf{S}(\mathbf{X}, t) \mathbf{N}$$

We can define the second Piola-Kirchhoff tensor  $\mathbf{S}$  given below:

$$\begin{aligned} \mathbf{S}(\mathbf{X}, t) &= J(\mathbf{X}, t) \mathbf{F}^{-1}(\mathbf{X}, t) \boldsymbol{\sigma}(\chi(\mathbf{X}, t), t) \mathbf{F}^{-T}(\mathbf{X}, t) \\ &= \mathbf{F}^{-1}(\mathbf{X}, t) \mathbf{P}(\mathbf{X}, t) . \quad \text{which is symmetric} \end{aligned}$$

Using Cauchy's relation:  $t_i(\mathbf{x}, t, \mathbf{n}) = \sigma_{ij}(\mathbf{x}, t) n_j$

and Nanson's formula:  $\mathbf{n} ds = J \mathbf{F}^{-T} \mathbf{N} dS$

# Continuum mechanics review: Dynamics

## Linearization of the Stress Tensors

It is important to check what are the effects of the Kinematic linearization on the three stress tensors.

From the relation:

$$\mathbf{P}\mathbf{F}^T = \mathbf{F}\mathbf{P}^T$$

We express the tensor  $\mathbf{P}$  in index form as:

$$P_{mk} = F_{mi}(P_{ij})^T (F_{jk})^{-T} = F_{mi}P_{ji}F_{kj}^{-1}$$

Using:  $F_{ij} = \delta_{ij} + \frac{\partial U_i}{\partial X_j}$  ;  $F_{ij}^{-1} = \delta_{ij} - \frac{\partial u_i}{\partial x_j}$

and  $\frac{\partial U_i}{\partial X_j} = \frac{\partial u_i}{\partial x_j} + O(\varepsilon^2) \simeq \frac{\partial u_i}{\partial x_j}$

$$P_{mk} = P_{km} - P_{jm} \frac{\partial U_k}{\partial X_j} + P_{ki} \frac{\partial U_m}{\partial X_i} - P_{ji} \frac{\partial U_m}{\partial X_i} \frac{\partial U_k}{\partial X_j}$$

Similarly the second Piolla-Kirchhoff stress tensor:

$$\mathbf{S} = \mathbf{F}^{-1} \mathbf{P}$$

takes the form in index notation:

$$S_{ij} = F_{ik}^{-1} P_{kj} = \left( \delta_{ik} - \frac{\partial U_i}{\partial X_k} \right) P_{kj} = P_{ij} - P_{kj} \frac{\partial U_i}{\partial X_k}$$

For the Cauchy stress tensor we express it using:

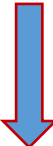
$$\mathbf{P} = J \boldsymbol{\sigma} \mathbf{F}^{-T}$$

as  $\sigma_{ij} = J^{-1} P_{ik} (F_{kj})^T = J^{-1} P_{ik} F_{jk}$

# Continuum mechanics review: Dynamics

## Linearization of the Stress Tensors

Using

$$\sigma_{ij} = J^{-1} P_{ik} (F_{kj})^T = J^{-1} P_{ik} F_{jk}$$
$$F_{ij} = \delta_{ij} + \frac{\partial U_i}{\partial X_j} ; F_{ij}^{-1} = \delta_{ij} - \frac{\partial u_i}{\partial x_j}$$
$$F_{ij} = \delta_{ij} + O(\varepsilon) ; F_{ij}^{-1} = \delta_{ij} - O(\varepsilon)$$
$$J \approx 1 + O(\varepsilon)$$

$$\sigma_{ij} = J^{-1} P_{ik} \left( \delta_{jk} + \frac{\partial U_j}{\partial X_k} \right)$$
$$= J^{-1} \left( P_{ij} + P_{ik} \frac{\partial U_j}{\partial X_k} \right) \approx P_{ij} + P_{ik} \frac{\partial U_j}{\partial X_k}$$

If we neglect the terms with the displacement gradient on the expressions:

$$P_{mk} = P_{km} - P_{jm} \frac{\partial U_k}{\partial X_j} + P_{ki} \frac{\partial U_m}{\partial X_i} - P_{ji} \frac{\partial U_m}{\partial X_i} \frac{\partial U_k}{\partial X_j}$$

$$S_{ij} = F_{ik}^{-1} P_{kj} = \left( \delta_{ik} - \frac{\partial U_i}{\partial X_k} \right) P_{kj} = P_{ij} - P_{kj} \frac{\partial U_i}{\partial X_k}$$

$$\sigma_{ij} = J^{-1} P_{ik} \left( \delta_{jk} + \frac{\partial U_j}{\partial X_k} \right)$$
$$= J^{-1} \left( P_{ij} + P_{ik} \frac{\partial U_j}{\partial X_k} \right) \approx P_{ij} + P_{ik} \frac{\partial U_j}{\partial X_k}$$



$$P_{mk} \approx P_{km} \quad S_{ij} \approx P_{ij} \quad \sigma_{ij} \approx P_{ij}$$